

Bridge Technical Committee Meeting Minutes

Date: Monday, March 20, 2023 Time: 1:00pm-3:00pm

Location: SWR Dane/Rock/Columbia Rm

Introductions

Online attendees: Jeremy Ashauer, Dominique Bechle, Bill Dreher, Brad Diener, Brent Freeman, Julie Brooks, Carolyn Brugman, Chad Halverson, Chandler Schreiber, Christine Hamil, Phil Ciha, Ruth Coisman, Mark Finnell, Gary Courneya, Brian Rowekamp, Joel Anderson (SPS), Habib Tabatabai, Hans Hallanger, Julie Jenks, Christine Krall, Linda Krueger, Leah Rhodes, Brandon Lamers, Luke Haun, Matt Grove, Mike Ryan, Dave Pantzlaff, Jim Parry, Pat Cashin, Cami Peterson, Craig Pringle, John Rublein, Ryan Pheifer, Elisabeth Stump, Dan Sydow, Ann Thielmann, Jonathan Thomas, Tim MacLaughlin-Barck, Tom Romenesko, Krissy VanHout, Craig Webster

In-Person attendees: Josh Dietsche, Aaron Bonk, Laura Shadewald, James Luebke, Dave Staab, Mark Mutziger, Kyle Busch, Kevin Weber, Isaac Groshek, Chad Hayes, Joe Balice, Josh Wade, Bill Ryan, Tadd Owens, David Stanke, Scott Stroud

Subcommittee Report(s)		10 <i>min</i>	
5 min	Design & Construction Subcommittee Update No specific requests came in from the contracting community since this last BTC meeting. Subcommittee will remain in place on an as-needed basis. No current plan in place for a meeting of this group.	Aaron Bonk	
5 min	Overlay Equipment Working Group James provided a brief background on the work that Terex Bidwell has been putting into this area, and further discussion will be held at the next meeting as that timing likely aligns better for more substantial updates.	James Luebke	
Standin	g Topics	25 min	

10 min	Wisconsin Highway Research Program Bridge Items http://wisconsindot.gov/Pages/about-wisdot/research/whrp.aspx James discussed two projects that recently completed their research: 1) Adhesive anchors and 2) Optimizing bridge slopes. James discussed active projects including low-slump concrete overlay improvements, best practices for underwater placement of concrete, and IR deck thermography. He also mentioned a project soon to kickoff, which will look into overhead and vertical concrete patching applications. Mark Mutziger asked if shotcrete would be investigated and Jim Parry asked if an APL will be developed. James responded that shotcrete and an APL are not the focus of the research, but are of interest.	Jame	s Luebke

10 min Bridge Manual Updates

James Luebke



<u>5 min</u>







happens one year (requiring netting) and the new build happens in the second year (requiring netting). The bid item is listed by station, not structure ID; but the intention is that netting is required in both situations. BOS can look into clarifying this issue by using a 'structure ID' and multiple "each" bid items when this situation presents itself.

Action Item(s): Aaron Bonk will follow up with WDNR, as well as BPD/BTS, to see if clarification guidance can be provided on all aspects of this issue.

- 5 min Jacking with Live Loads/Staged Construction James followed up with the BOS repair crews to see what their experience has been with jacking under live loads. They indicated that they view this as a case-by-case issue, the structure type dictates whether this can be handled or not, and that they feel that treating each case on a project-level basis is the best course of action. There doesn't appear to be a clear direction that can be applied uniformly for all projects. David Stanke asked whether the loads would be provided when jacking is required per plan, and James indicated that existing dead and live load reactions are to be placed on the plans. This is a change from what has been the case in the past. No further action required at this time (item to be closed).
- 5 min Exposure of Epoxy Reinf. In P/S Girders At the last meeting, it was discussed that bags were used on every shear stirrup on girders to keep the 60-day exposure window requirement in place per the standard spec. James reviewed this situation by researching other states' requirements, what the spec requirements are (ASTM's), etc.; and there is no indication that WisDOT is being overly conservative by keeping this requirement. James also stated that some other states have been using reusable covers, which may be more environmentally conscious compared to the bags currently being used. No further action required at this time (item to be closed).

New Topics

10 min Seal Concrete Mix Design

On a recent project, discussions between the contractor and the project team about requirements for seal concrete were brought forward. The specific issue at hand was the nature of a given seal for a project – "non-structural" vs. "structural" – and whether modifications to the mix design could be made. BOS and BTS approved the request to change the aggregate size for



James Luebke

40 min

Aaron Bonk/Jim Parry







this particular project given that the seals were "nonstructural" in nature given that piles were driven through the seal to bearing, and that the seals were there to be able to dewater and pour the footings and piers above. Discussion held on the need to have two separate seal bid items in the spec was held. Kevin Weber indicated that allowing the contractor to determine what types of mixes to use would be beneficial. Kevin also commented on the inability to pour and pump No. 2 fractured stone, whether it is high or low slump, in these types of pours. Granite/limestones that are used in the northern part of the state are difficult to pour in this fashion. David Stanke indicated that he has seen the same thing, and that the main issue is that inconsistencies amongst field staff are what cause risk to their bids (they don't know what to expect from field staff).

The discussion led to the main question being whether a No. 2 stone is needed in seal pours or not (and other structure pours as well), especially when the rebar spacing is not allowing vibrators to get in to stop segregating the concrete. Mark Finnell stated that a recent spec change in 501 was made to allow concrete masonry seals to use the No. 1 stone. Jim Parry indicated that optimized mixes can eliminate the need for the larger stone, and also will aid in reducing cracking which was the original intent of the larger size aggregate in the older mix designs.

Action Item(s): Aaron Bonk will set up discussions with BTS and Krissy VanHout to determine if additional spec modifications should be considered to aid in avoiding future field contract administration disputes.

10 min Opening Concrete/Maturity Method

On a recent project, a contractor was using the maturity method in order to streamline getting their forms stripped/pulled. On this particular project, the cylinders were being cured offsite in a controlled environment, however, they were instructed that the cylinders were supposed to be cured in a similar fashion to the rest of the structure. When the contractor pulled those cylinders back to the elements similar to the field, they were unable to get strengths on their breaks. Kevin was wondering whether other contractors have been running into similar direction for this type of situation as it is inconsistent with his past knowledge of this method. David Stanke indicated that he didn't understand the WisDOT direction to field cure cylinders with the maturity method, as most projects they've worked on for WisDOT and the rest of Kevin Weber



updates to the FDM/CMM. Action Item(s): Hans Hallanger to review this situation and provide a status update at a future

Bridge Tech Committee meeting.

JREAU OF







5 min In-Person vs. Hybrid Bridge Tech Meetings Moving Forward Aaron discussed whether it would be beneficial to get attendees back in person for all meetings, some meetings, etc. Short discussions were held with the recommendation for BOS to send out a poll to attendees. Action Item(s): Aaron Bonk will send out a poll to attendees to see what their preferences are for this meeting moving forward.

5 min Major Specification Rewrite/Movement Action Item(s): Mark will bring forward a topic at the next meeting for more widespread awareness. Mark Finnell





Bridge Technical Committee Meeting Sign-In Sheet

Date: Monday, March 20, 2023

Time: 1:00pm-3:00pm

Location: SWR Dane/Rock/Columbia Rm

Name	Company	Email
AARON BONK	WISDOT BOS	AMEN, BOUKE DOT. WI. GOV
TADD OWENS	Corré	towense correinc. Com
Bill Ryan	Goncrefe Structures Inc	WPTPCSinc-Wi-com
Josh Wade	Arbor Green	Josh wade Carborgreen wi com
Joe Balice	FHWA	joe-balice e dot.gov
Scott Stroud	Zenith Tech	Stroud @ wallsegatoup.com
MARK NUNTZIGER	COLLINS	mont 2: ger Q colling dugr. com
Preston Moore	County Materials	preston. moore @ countymaterials. C.
Lyle Busch	N'3DOT BOS	Kelebusch (dot.m.gov
DAVID STAAB	WIS DOT BTS	durid. Stand C dot wigger
CHAP HAYES	WISDOT BRD	
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Kaizn wassin	515	husbardestres trunk set thes.com
JAMES LUEDRE	WISDOT BOS	Jamos. Webke Pot. Wigen
Laum Shadawa (el	n	Jana, Shudewald Odot, wigor
JOSH DIBIJCHE	11 <i>L</i> i	JOSHUA. DIETSCHE DOT. WI, GOV
DAUJO STANKE	KRAEMER NORTH AMERICA	DISTANKER KRAEMER NA. COM
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WisDOT Bridge Manual January 2023 Updates

James Luebke, P.E. Policy and Standards Engineer March 1, 2023

• Chapter 18 – Slab Falsework Removal



Standards: 18.01, 18.02, 18.03





- Chapter 19 Lifting Checks
 - Clarified:
 - Span < 153 ft → Lifting check <u>not required</u>
 - Span ≥ 153 ft → Noted as a Long Span and lifting check required at 1/10 points

THIS NOTE APPLIES TO LONG SPANS AS DEFINED IN THE NOTES FOR THE 72W" GIRDER, TABLE 19.3-2 OF THE BRIDGE MANUAL: FOR STORAGE, HANDLING, AND TRANSPORTING, THIS GIRDER IS REINFORCED TO ALLOW A MAXIMUM OVERHANG FROM THE LIFTING LOCATION OR POINT OF SUPPORT OF UP TO 1/10 THE GIRDER LENGTH. THE CONTRACTOR IS RESPONSIBLE FOR LATERAL STABILITY OF THE GIRDER UNTIL THE DECK IS CURED. (IF NOTE DOESN'T APPLY, REFERENCE SECT. 503.3.4 OF STD. SPEC. FOR GUIDANCE)





Chapter 39 – Contractor Designed Structures Clarified anchor rods are contractor designed







- Chapter 40 Staged Temporary Support
 - Added guidance for deck replacements with staged construction.
 - Evaluate Temporary Support Condition
 - Use Temporary Support SPV bid item for contractor-designed support







Other Updates

 New Approved Products List (APL) and Updated STSP:
Sheet Membrane Waterproofing for Asphalt Overlays (516.0600.S) < 6-inches between membrane and asphaltic pavement
Sheet Membrane Waterproofing for Buried Structures (516.0610.S) > 6-inches between membrane and asphaltic pavement







Other Updates

• Updated BOS Special Provisions:

- Temporary Support 02/23
- Temporary Bridge Widening 02/23
- Temporary Structure 02/23

Bureau of Structures	Design & Co	nstruction			
Design & Construction	Policy Memos	Bridge Manual Special Provisions Standard Bridge Design Tool Survey Reports & O	hecklists		
Maintenance & Inspection	Structure Costs Plan Submittal Bridge Technical Committee Construction Resources Contacts				
Fabrication & Ouality	File	Description	Updated		
Assurance	Adjusting Diaphragms	This special provision describes adjusting diaphragms and cross frames where girders are vertically realigned. Perform the work in accordance with section 506 of	04/18		
Manuals & HSI Quick	a september of	the standard specifications, as shown on the plans, and as hereinafter provided.			
Links Research & Outreach	Asphalt Panels	This special provision describes furnishing and placing asphalt protection in accordance with Chapter 29, Part 2 of the American Railway Engineering and Maintenance-of-Way Association (AREMA) Manual, as shown on the plans, and as hereinafter provided.	04/18		
	Bearing Maintenance	This special provision describes removing the expansion bearings at the abutments and piers, blast cleaning and painting them, and reinstalling the bearings, in accordance with the plans and as hereinafter provided.	04/18		
	Repair	This special provision describes removing the bearings at the piers, blast cleaning and priming the bearings, furnishing and placing shims, resetting the bearings, and furnishing and placing new anchor bolts. All work shall be in accordance with the pertinent provisions of the standard specifications, the plans, and as hereinafter provided.	04/18		
	Boulder Retards	This special provision describes furnishing and placing boulder retards in a given waterway.	04/18		





Questions and/or Feedback

Contact: James Luebke, P.E. Policy and Standards Engineer 608-266-5098 James.Luebke@dot.wi.gov





WisDOT Division of Transportation Systems Development Bureau of Technical Services Quality Assurance – Concrete Unit 3502 Kinsman Boulevard Madison, WI 53704 Governor Tony Evers Secretary Craig Thompson wisconsindot.gov



March 20, 2023

MEMORANDUM: Concrete Maturity: Requirements and Guidance

Background:

The department offers several ways to determine opening strength for concrete. One of the methods that can be used is maturity. Maturity is the concept of measuring the concrete's temperature and the time it takes to reach that temperature. This time-temperature relationship can then be used to estimate the concrete's strength when cylinders or beams are broken at certain times. This method provides a non-destructive means of estimating the in-situ concrete's strength and determine if the concrete has gained enough strength to open it to service or removing formwork and falsework.

Requirements for Calibrating Maturity Curves:

CMM 870.4.8.1 and CMM 870.4.8.2 outlines the requirements for calibrating maturity curves for WisDOT contracts. A maturity curve must be developed for each mix design that will use this method to determine opening strength. The procedure for developing and calibrating the maturity curve requires the calibration samples being made in the field instead of being made in the lab. The cylinders used to develop the curve are casted on the first day of the concrete pour on the project. After the cylinders have been casted, they need to be cured in similar conditions to the concrete in the field. This means placing these cylinders in with the concrete that was placed so the cylinders can cure under the same environmental conditions. It has been shown that calibrating maturity curves using this method matches with how the concrete is curing in the field. Standard Specification 502.3.10.1.3.3 requires the maturity curve to have data points that exceed 120 percent or greater than the required opening strength. What this means is the curve only needs to be developed for the duration it takes the concrete strength to exceed 120 percent of the required opening strength specified in the contract. Once the data points reach or exceed the 120 percent threshold and meets the other remaining requirements found in ASTM C1074, the maturity curve can be used to estimate the opening strength on the project. Submit the newly established maturity curve to WisDOT project staff for review and approval prior to use.

Requirements for Verifying Concrete Maturity:

Standard Specification 502.3.10.1.3.3 outlines the verification of the maturity curve and when a new one must be developed. A set of verification cylinders must be made each work week to validate the curve. These verification cylinders must be field cured alongside the concrete element that was casted. This curing process is not dissimilar to how the calibration cylinders were cured during development of the curve. The verification cylinders that are broken must break within +/- 10% of the curve. If the verification cylinders vary greater than +/- 10%, the maturity curve must be redone. Similarly, a new maturity curve must be developed if the concrete mixture design changes. Any changes to the mix design may affect how the concrete reacts which may impact the time-temperature relationship. Development of the new curve will have to follow the same procedures as the previous curve.

Guidance on Concrete Maturity:

Concrete maturity is a good way to estimate the in-situ concrete strength. However, there are items to consider when using maturity. One of those items is the placement of the maturity sensor(s) within the concrete element. CMM 870.4.8.3 states to install sensors in locations that are critical in terms of exposure conditions and structural requirements. One such critical area are the edges of the concrete element. Concrete at the edges will cure slower than concrete in the center of the element. It is good practice to place sensors 2" - 4" from the edge of the element. Another item to consider with maturity are seasonal changes. A maturity curve developed during colder weather will not be valid during warmer weather. The hydration (reaction) of cement can be impacted by temperature and humidity changes which has a direct impact on the maturity curve. A new maturity curve will be needed to reflect with the change of seasons. In addition to seasonal changes, the maturity sensors should not be turned on until they have been embedded in the mass of concrete or the verification cylinders. Premature activation of these sensors can impact the maturity calculation and provide inaccurate data. The next item to consider is to have multiple maturity sensors placed in the concrete. Though 502.3.10.1.3.3 specifies the minimum number of sensors to be placed in the concrete, it is good practice to place an extra sensor just in case of sensor failure. This extra sensor should be placed in a different critical location. Additionally, having more sensors will help build a better temperature profile of the concrete element to ensure different parts of the element are curing at the same rate. The last item to consider is to cast a separate set of opening strength cylinders. Doing this can help supplement the maturity curve if there are sensor failures or if the verification cylinders exceed the +/- 10% threshold.